

NOTICE OF PROPOSED DEVELOPMENT

Notice is hereby given that an application has been made for planning approval for the following development:

SITE:**64 FRIENDSHIP DRIVE, SORELL****PROPOSED DEVELOPMENT:****DWELLING**

The relevant plans and documents can be inspected at the Council Offices at 47 Cole Street, Sorell during normal office hours, or the plans may be viewed on Council's website at www.sorell.tas.gov.au until **Monday 25th August 2025**.

Any person may make representation in relation to the proposal by letter or electronic mail (sorell.council@sorell.tas.gov.au) addressed to the General Manager. Representations must be received no later than **Monday 25th August 2025**.

APPLICATION NO: 5.2025-194.1
DATE: 06 AUGUST 2025

Part B: Please note that Part B of this form is publicly exhibited.

Full description of Proposal:	Use: Dwelling
	Development: Construction of a single dwelling
	<i>Large or complex proposals should be described in a letter or planning report.</i>
Design and construction cost of proposal:	\$ 350,000.00

Is all, or some the work already constructed:	No: ☐ Yes: ☒
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Location of proposed works:	Street address: 64 Friendship Drive
	Suburb: SORELL Postcode: 7172
	Certificate of Title(s) Volume: 185351 Folio: 322

Current Use of Site	Vacant Land
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Current Owner/s:	Name(s) Lynmore Holdings Pty Ltd
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Is the Property on the Tasmanian Heritage Register?	No: ☒ Yes: ☐	<i>If yes, please provide written advice from Heritage Tasmania</i>
Is the proposal to be carried out in more than one stage?	No: ☒ Yes: ☐	<i>If yes, please clearly describe in plans</i>
Have any potentially contaminating uses been undertaken on the site?	No: ☒ Yes: ☐	<i>If yes, please complete the Additional Information for Non-Residential Use</i>
Is any vegetation proposed to be removed?	No: ☒ Yes: ☐	<i>If yes, please ensure plans clearly show area to be impacted</i>
Does the proposal involve land administered or owned by either the Crown or Council?	No: ☒ Yes: ☐	<i>If yes, please complete the Council or Crown land section on page 3</i>

If a new or upgraded vehicular crossing is required from Council to the front boundary please complete the Vehicular Crossing (and Associated Works) application form
<https://www.sorell.tas.gov.au/services/engineering/>

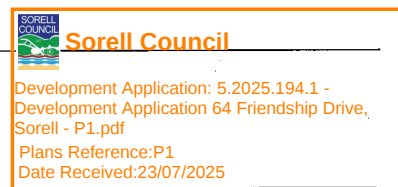


Sorell Council

Development Application: 5.2025.194.1 -
 Development Application 64 Friendship Drive,
 Sorell - P1.pdf
 Plans Reference: P1
 Date Received: 23/07/2025

Declarations and acknowledgements	
- I/we confirm that the application does not contradict any easement, covenant or restriction specified in the Certificate of Title, Schedule of Easements or Part 5 Agreement for the land. - I/we consent to Council employees or consultants entering the site and have arranged permission and/or access for Council's representatives to enter the land at any time during normal business hours. - I/we authorise the provision of a copy of any documents relating to this application to any person for the purposes of assessment or public consultation and have permission of the copyright owner for such copies. - I/we declare that, in accordance with s52(1) of the <i>Land Use Planning and Approvals Act 1993</i>, that I have notified the owner(s) of the intention to make this application. - I/we declare that the information in this application is true and correct. <i>Details of how the Council manages personal information and how you can request access or corrections to it is outlined in Council's Privacy Policy available on the Council website.</i>	
I/we acknowledge that the documentation submitted in support of my application will become a public record held by Council and may be reproduced by Council in both electronic and hard copy format in order to facilitate the assessment process, for display purposes during public exhibition, and to fulfil its statutory obligations. I further acknowledge that following determination of my application, Council will store documentation relating to my application in electronic format only.	
Where the General Manager's consent is also required under s.14 of the <i>Urban Drainage Act 2013</i>, by making this application I/we also apply for that consent.	
Applicant Signature:	Signature: Date: 23/07/2025

Crown or General Manager Land Owner Consent	
If the land that is the subject of this application is owned or administered by either the Crown or Sorell Council, the consent of the relevant Minister or the Council General Manager whichever is applicable, must be included here. This consent should be completed and signed by either the General Manager, the Minister, or a delegate (as specified in s52 (1D-1G) of the <i>Land Use Planning and Approvals Act 1993</i>). Please note: - If General Manager consent is required, please first complete the General Manager consent application form available on our website www.sorell.tas.gov.au - If the application involves Crown land you will also need a letter of consent. - Any consent is for the purposes of making this application only and is not consent to undertaken work or take any other action with respect to the proposed use or development.	
I _____ being responsible for the administration of land at _____ declare that I have given permission for the making of this application for _____	
Signature of General Manager, Minister or Delegate:	Signature: Date:



DOYLE **SOIL** **CONSULTING**



SITE AND SOIL EVALUATION REPORT **FOUNDATION AND WINDLOADING ASSESSMENT**

64 (Lot 322) Friendship Drive

Sorell

March 2025



Sorell Council

Development Application: 5.2025.194.1 -
Development Application 64 Friendship Drive,
Sorell - P1.pdf

Plans Reference: P1
Date Received: 23/07/2025

Doyle Soil Consulting: 6/76 Auburn Rd Kingston Beach 7050 – 0488 080 455 – robyn@doylesoilconsulting.com.au

SITE INFORMATION

Client: Creative Homes Hobart

Address: Lot 322 Friendship Drive, Sorell (CT 185351/322)

Site Area: Approximately 580 m²

Date of inspection: 12/03/2025

Building type: New house(s)

Services: Mains water and sewer

Relevant Planning Overlays: None

Mapped Geology - Mineral Resources Tasmania 1:50 000 Sorell sheet: **Tb** = Basalt (tholeiitic to alkalic) and related pyroclastic rocks

Soil Depth: 0.8 – 1.1 m

Subsoil Drainage: Well drained

Drainage lines/water courses: None

Vegetation: pasture

Rainfall in previous 7 days: Approximately 0 mm

Slope: Approximately 5° to the west

SITE ASSESSMENT AND SAMPLE TESTING

Site investigation and soil classification in accordance with AS 2870-2011 *Residential slabs and footings* and in accordance with AS 4055-2021 *Wind load for Housing*. Test holes were dug using a Christie Post Driver Soil Sampling Kit, comprising a CHPD78 Christie Post Driver with a Soil Sampling Tube (50 mm OD x 1600/2100 mm). For test hole and DCP locations, see Appendix 1.

- Two test hole (TH) cores:
 - TH1 (B) with refusal at 1.1 m
 - TH2 (C) with refusal at 0.8 m
- One Dynamic Cone Penetrometer (DCP) test:
 - DCP1 with refusal at 0.9 m
- Emerson Dispersion test on subsoils and linear shrinkage tests on all likely founding layers.

SOIL PROFILES – Test Hole B and C

Depth (m) TH-B	Depth (m) TH-C	Horizon	Description and field texture grade	USCS Class
0 – 0.5	0 – 0.2	FILL	Uncontrolled, dominantly local highly reactive clay FILL	CH
0.5 – 1.1	0.2 – 0.8	B1/B2	Black (10YR 2/1), Silty Medium Clay , massive/compacted grading to weak coarse angular blocky at depth, dry hard grading to slightly moist stiff to hard consistency. Refusal on basalt bedrock.	CH



SITE AND SOIL COMMENTS

Layers of uncontrolled local highly reactive clay fill are present on the site to maximum observed depths of 0.2 to 0.5 m.

The *natural* soil profiles (excluding fill and disturbed materials) are formed from clayey colluvium derived from Tertiary basalt. The profiles are moderately deep with refusal occurring at approximately 0.8 to 1.1 m. The field textures of the soil profile are dominated by clay, which is highly reactive, weakly structured and mildly dispersive. The DCP indicates a low bearing capacity to at least 0.7 m. Founding on the underlying, highly competent, basalt bedrock, at approximately 0.8 to 1.1 m depth, is recommended.

Very common 5-10 cm wide cracks present at soil surface indicative of highly reactive clays.

LINEAR SHRINKAGE AND SOIL REACTIVITY

Samples of the clayey subsoils were tested for reactivity using the linear shrinkage test. Linear shrinkage provides an approximate guide to aid site classification (for foundations) based on the reactivity of clays. The results suggest the clays are highly reactive (refer to tables below and *AS2870-2011 clause 2.1.2 table 2.1*).

TH #	Depth (m)	Length of mould (mm)	Longitudinal Shrinkage (LS) in mm	LS (%)	Soil Class
B + C	B1/B2	125	23	18.4	H – 1

DCP TESTS AND ESTIMATED BEARING CAPACITY

A minimum bearing capacity of 100 kPa is required for strip and pad footings and under the edge footings and associated slab foundations (refer to tables below and *AS2870-2011 clause 2.4.5*). We provide estimated soil bearing strengths along with a variance range (+/-) based on a review of published literature relating field Dynamic Cone Penetrometer (DCP) readings to triaxial soil strength tests.

DCP testing is a method of estimating likely soil bearing capacity. However, surface layers (upper ~0.7 m) are subject to seasonal variation in soil moisture content, leading to possible higher DCP values in summer/drought conditions. Moisture-related variability in soil bearing capacity is most pronounced in coherent soils – i.e., clays and silty clays. These may be very stiff

or hard when dry, while only soft to firm when moist/slightly moist - refer to *soil consistency* in the above profile descriptions).

Soil moisture below ~0.7 m will vary less with the season, meaning DCP values; hence, soil-bearing capacity at these depths is likely to be representative of year-round conditions.

When estimating the suitable foundation depth, we take into account the interplay between soil bearing capacity and seasonally variable soil moisture conditions in the upper layers. The subsoils in the upper 0.7 m were slightly moist when tested (March '25).

The data from DCP1 indicate the bearing capacity of the soil is at a *suitable* strength below 0.7 m. However, the highly competent basalt bedrock at approximately 0.9 m would be the *recommended* foundation material.

Based on the DCP data and core depths, the recommended foundation depth can range from approximately 0.8 to 1.1 m.

DCP 1				
Depth (mm)	DCP n-number (Blows/100 mm)	DCP Penetration Index (mm/Blow)	Estimated Bearing Capacity (kPa = n x 30)	Likely Variance (+/-)
0 - 100	5	20.0	150	50
100 - 200	15	6.7	450	150
200 - 300	9	11.1	270	90
300 - 400	12	8.3	360	120
400 - 500	13	7.7	390	130
500 - 600	9	11.1	270	90
600 - 700	7	14.3	210	70
700 - 800	13	7.7	390	130
800 - 900	30	3.3	900	300

EMERSON AGGREGATE DISPERSION TEST

Soils with an excess of exchangeable sodium ions on the cation exchange complex (clays), can cause clay dispersion. Under some circumstances, the presence of dispersive soils can also lead to significant erosion, and in particular, tunnels leading to eventual gully erosion. Dispersive clay subsoil materials can also cause sealing of the soil surface – if left out in wet weather, they then dry and set very hard in dry weather. A field survey of the property and the surrounding area found no erosion due to soil dispersion.

The subsoil was tested for dispersion using the Emerson Aggregate Test (EAT). Testing resulted in Emerson class 2(2), indicating presence of soils with mild dispersion characteristics. As such, exposure to rainfall may lead to spontaneous clay dispersion and erosion, as previously discussed.

To minimise this, we recommend coverage of exposed subsoil with topsoil or regular treatment with gypsum at 0.5 Kg/m² along with minimising subsoil disturbance whenever possible.

TH #	Depth (m)	Visual sign	Class
B + C	B1	Some dispersion (obvious milkiness < 50% of aggregate affected)	2(2)

WIND CLASSIFICATION

The following wind classification for the site is in accordance with AS 4055-2021 (*Wind loads for Housing*). For structures other than class 1 and class 10 structures, or that exceed the geometric limits in Clause 1.2 of AS 4055-2021, the wind classification shall be calculated in accordance with AS 1170.2-2021 (*Structural Design Actions – Wind Actions*).

The wind classification for the site, per AS 4055-2021:

Region:	A
Terrain Category:	TC1 – treeless paddocks
Shielding Classification:	PS – 2nd row of houses from edge of development
Topographic Classification:	T1 – middle 3rd of 1:12 slope feature
Wind Classification:	N2
Design Wind Gust Speed ($V_{h,u}$):	40 m/sec

SITE CLASSIFICATION AND RECOMMENDATIONS

For standard foundations (100 kPa bearing capacity), the site meets the criteria for a **Class P** site classification, as set out in AS2870-2011 (construction). This classification is appropriate due to the presence of (a) uncontrolled fill to maximum observed depths of approximately 0.5 m, and (b) materials with variable and low bearing capacity to approximately 0.7 m depth. Founding on the underlying and highly competent basalt bedrock, at approximately 0.8 – 1.1 m depth, is recommended.

If a stiffened raft-type foundation is utilised (where slab is tied to edge footings), the bearing capacity requirement is reduced to 50 kPa, and therefore a site classification of **Class M** becomes appropriate (See note 1). However, we strongly recommend founding on the underlying highly competent bedrock at approximately 0.8 – 1.1 m.

Note 1 – In addition to the **Class P** site classification, the clay layers meet the reactivity levels of Class M or moderately reactive, with 20 – 40 mm the dominant reactivity of expected surface movement under normal soil moisture ranges for the location.

Note 2 – If founded entirely on underlying competent basalt bedrock (recommended), below approximately 0.8 to 1.1 m, and no part of the foundations, be it a slab, pier or footing, is in contact with/or is supported by the subsoils, then **Class M** would become an appropriate site classification.

Note 3 – All foundations require ongoing adequate drainage and vegetation management – please refer to the attached CSIRO foundation management BTF 18 sheet.

Note 4 – If any foundations are placed on FILL that is > 0.5 m in depth, then **Class P** is applicable.

Note 5 – Based on the upper 0.6 m of soil, all plumbing fixtures and fittings should be installed using **Class M** as per *Appendix G AS/NZS 3500.2.2021*.

General Notes – Important points pertinent to the maintenance of foundation soil conditions

This report relates to the soil and site conditions on the property at the time of the site assessment. The satisfactory long-term performance of footings is dependent upon ongoing site maintenance by the owner.

Examples of abnormal moisture conditions developing after construction include the following:

- A) The effect of trees too close to the footings.
- B) Excessive or irregular watering of gardens adjacent to the footings.
- C) Failure to maintain site drainage affecting footings.
- D) Failure to repair plumbing leaks affecting footings.
- E) Loss of vegetation from near the building.

All earthworks on site must comply with AS 3798-2007 Guidelines on Earthworks for commercial and residential developments.

REPORT LIMITATIONS

Whilst every attempt is made to describe sub-surface conditions, natural variation will occur that cannot be determined by limited investigative soil testing. Therefore, discrepancies are possible between test results and observations during construction. It is our intention to accurately indicate the most probable soil type(s) and conditions for the area assessed. However, due to the nature of sampling an area, variations in soil type, soil depth and site conditions may occur.

We accept no responsibility for any differences between what we have reported and actual site and soil conditions for particular regions we could not directly assess at the time of inspection.

It is recommended that during construction, Doyle Soil Consulting and/or the design engineer be notified of any major variation to the foundation conditions as predicted in this report. Any changes to the site through excavations may alter the site classification.

In these cases, it is expected that the owner consults the author for a reclassification. This report requires certification via a form 55 certificate from Doyle Soil Consulting to validate its contents.

Because site discrepancies may occur between this report and actual site conditions, it is a condition of certification of this report that the builder be provided with a copy of this report.



Rowan Mason
B.Agr.Sc.(Hons).
Soil Scientist



Dr. Richard Doyle
B.Sc.(Hons), M.Sc.(Geol), Ph.D. (Soil Sci.),
CPSS (Certified Prof Soil Scientist)
Geologist and Soil Scientist



APPENDIX 1 – Approximate test hole and DCP locations



APPENDIX 2 – Definitions of Soil Horizons

Horizon name	Meaning
A1	Dark topsoils, zone of maximum organic activity
A2 or E	Leached, light/pale washed-out sandy layer
A3 or AB	Transition from A to B, more like A
B1 or BA	Transition from A to B, more like B
B2	Main subsoils layer with brown colouration, accumulations of clay, humus, iron oxide, etc
B3	Transitional from B2 to C
C	Weakly weathered soil parent materials
Subscript	Meaning
r	Reducing conditions (anaerobic)
t	Enriched in translocated clay
s	Iron/aluminium oxide accumulations in subsoil
g	Mottled, suggesting periodic/seasonal wetness
m	Cemented layer (oxides, carbonates, humus, silica etc)
k	Calcium carbonate (lime) accumulation
h	Humus accumulation in subsoil

CERTIFICATE OF QUALIFIED PERSON – ASSESSABLE ITEM

Section 321

Form **55**

To: Owner name
 Address
 Suburb/postcode

Qualified person details:

Qualified person:
Address: Phone No:
 Fax No:
Licence No: Email address:

Qualifications and Insurance details:
(description from Column 3 of the Director's Determination - Certificates by Qualified Persons for Assessable Items)

Speciality area of expertise:
(description from Column 4 of the Director's Determination - Certificates by Qualified Persons for Assessable Items)

Details of work:

Address: Lot No:
 Certificate of title No:
The assessable item related to this certificate:
(description of the assessable item being certified)
Assessable item includes –
- a material;
- a design
- a form of construction
- a document
- testing of a component, building system or plumbing system
- an inspection, or assessment, performed

Certificate details:

Certificate type:
(description from Column 1 of Schedule 1 of the Director's Determination - Certificates by Qualified Persons for Assessable Items n)

This certificate is in relation to the above assessable item, at any stage, as part of - (tick one)

building work, plumbing work or plumbing installation or demolition work: ☒

or

a building, temporary structure or plumbing installation: ☐

In issuing this certificate the following matters are relevant –

Documents:

The attached Geotechnical Assessment Report for the address detailed above in, 'Details of Work'.

Relevant
calculations:

Refer to above report.

References:

AS1726-2017 Geotechnical site investigations

CSIRO Building Technology File -18

Substance of Certificate: (what it is that is being certified)

Geotechnical Assessment -Site and soil classification

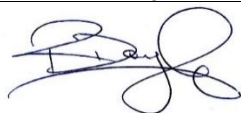
Scope and/or Limitations

The classification applies to the site as inspected and does not account for future alteration to foundation conditions as a result of earthworks, drainage condition changes or variations in site maintenance.

I certify the matters described in this certificate.

Qualified person:

Signed:



Certificate No:

1700

Date:

26/03/2025



Foundation Maintenance and Footing Performance: A Homeowner's Guide



PUBLISHING

BTF 18-2011
replaces
Information
Sheet 10/91

Buildings can and often do move. This movement can be up, down, lateral or rotational. The fundamental cause of movement in buildings can usually be related to one or more problems in the foundation soil. It is important for the homeowner to identify the soil type in order to ascertain the measures that should be put in place in order to ensure that problems in the foundation soil can be prevented, thus protecting against building movement.

This Building Technology File is designed to identify causes of soil-related building movement, and to suggest methods of prevention of resultant cracking in buildings.

Soil Types

The types of soils usually present under the topsoil in land zoned for residential buildings can be split into two approximate groups – granular and clay. Quite often, foundation soil is a mixture of both types. The general problems associated with soils having granular content are usually caused by erosion. Clay soils are subject to saturation and swell/shrink problems.

Classifications for a given area can generally be obtained by application to the local authority, but these are sometimes unreliable and if there is doubt, a geotechnical report should be commissioned. As most buildings suffering movement problems are founded on clay soils, there is an emphasis on classification of soils according to the amount of swell and shrinkage they experience with variations of water content. The table below is Table 2.1 from AS 2870-2011, the Residential Slab and Footing Code.

Causes of Movement

Settlement due to construction

There are two types of settlement that occur as a result of construction:

- Immediate settlement occurs when a building is first placed on its foundation soil, as a result of compaction of the soil under the weight of the structure. The cohesive quality of clay soil mitigates against this, but granular (particularly sandy) soil is susceptible.
- Consolidation settlement is a feature of clay soil and may take place because of the expulsion of moisture from the soil or because of the soil's lack of resistance to local compressive or shear stresses. This will usually take place during the first few months after construction, but has been known to take many years in exceptional cases.

These problems are the province of the builder and should be taken into consideration as part of the preparation of the site for construction. Building Technology File 19 (BTF 19) deals with these problems.

Erosion

All soils are prone to erosion, but sandy soil is particularly susceptible to being washed away. Even clay with a sand component of say 10% or more can suffer from erosion.

Saturation

This is particularly a problem in clay soils. Saturation creates a bog-like suspension of the soil that causes it to lose virtually all of its bearing capacity. To a lesser degree, sand is affected by saturation because saturated sand may undergo a reduction in volume, particularly imported sand fill for bedding and blinding layers. However, this usually occurs as immediate settlement and should normally be the province of the builder.

Seasonal swelling and shrinkage of soil

All clays react to the presence of water by slowly absorbing it, making the soil increase in volume (see table below). The degree of increase varies considerably between different clays, as does the degree of decrease during the subsequent drying out caused by fair weather periods. Because of the low absorption and expulsion rate, this phenomenon will not usually be noticeable unless there are prolonged rainy or dry periods, usually of weeks or months, depending on the land and soil characteristics.

The swelling of soil creates an upward force on the footings of the building, and shrinkage creates subsidence that takes away the support needed by the footing to retain equilibrium.

Shear failure

This phenomenon occurs when the foundation soil does not have sufficient strength to support the weight of the footing. There are two major post-construction causes:

- Significant load increase.
- Reduction of lateral support of the soil under the footing due to erosion or excavation.

In clay soil, shear failure can be caused by saturation of the soil adjacent to or under the footing.

GENERAL DEFINITIONS OF SITE CLASSES

Class	Foundation
A	Most sand and rock sites with little or no ground movement from moisture changes
S	Slightly reactive clay sites, which may experience only slight ground movement from moisture changes
M	Moderately reactive clay or silt sites, which may experience moderate ground movement from moisture changes
H1	Highly reactive clay sites, which may experience high ground movement from moisture changes
H2	Highly reactive clay sites, which may experience very high ground movement from moisture changes
E	Extremely reactive sites, which may experience extreme ground movement from moisture changes

Notes

1. Where controlled fill has been used, the site may be classified A to E according to the type of fill used.
2. Filled sites. Class P is used for sites which include soft fills, such as clay or silt or loose sands; landslide; mine subsidence; collapsing soils; soil subject to erosion; reactive sites subject to abnormal moisture conditions or sites which cannot be classified otherwise.
3. Where deep-seated moisture changes exist on sites at depths of 3 m or greater, further classification is needed for Classes M to E (M-D, H1-D, H2-D and E-D).

Tree root growth

Trees and shrubs that are allowed to grow in the vicinity of footings can cause foundation soil movement in two ways:

- Roots that grow under footings may increase in cross-sectional size, exerting upward pressure on footings.
- Roots in the vicinity of footings will absorb much of the moisture in the foundation soil, causing shrinkage or subsidence.

Unevenness of Movement

The types of ground movement described above usually occur unevenly throughout the building's foundation soil. Settlement due to construction tends to be uneven because of:

- Differing compaction of foundation soil prior to construction.
- Differing moisture content of foundation soil prior to construction.

Movement due to non-construction causes is usually more uneven still. Erosion can undermine a footing that traverses the flow or can create the conditions for shear failure by eroding soil adjacent to a footing that runs in the same direction as the flow.

Saturation of clay foundation soil may occur where subfloor walls create a dam that makes water pond. It can also occur wherever there is a source of water near footings in clay soil. This leads to a severe reduction in the strength of the soil which may create local shear failure.

Seasonal swelling and shrinkage of clay soil affects the perimeter of the building first, then gradually spreads to the interior. The swelling process will usually begin at the uphill extreme of the building, or on the weather side where the land is flat. Swelling gradually reaches the interior soil as absorption continues. Shrinkage usually begins where the sun's heat is greatest.

Effects of Uneven Soil Movement on Structures

Erosion and saturation

Erosion removes the support from under footings, tending to create subsidence of the part of the structure under which it occurs. Brickwork walls will resist the stress created by this removal of support by bridging the gap or cantilevering until the bricks or the mortar bedding fail. Older masonry has little resistance. Evidence of failure varies according to circumstances and symptoms may include:

- Step cracking in the mortar beds in the body of the wall or above/below openings such as doors or windows.
- Vertical cracking in the bricks (usually but not necessarily in line with the vertical beds or perpend).

Isolated piers affected by erosion or saturation of foundations will eventually lose contact with the bearers they support and may tilt or fall over. The floors that have lost this support will become bouncy, sometimes rattling ornaments etc.

Seasonal swelling/shrinkage in clay

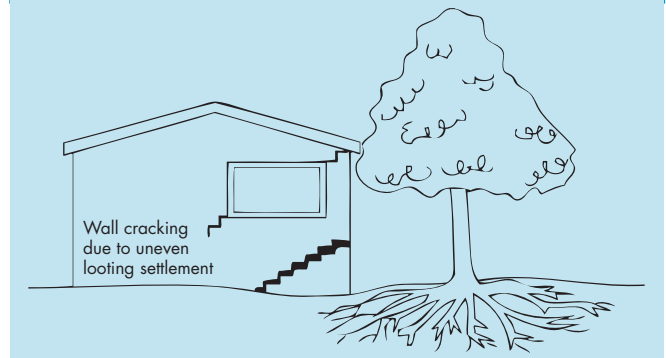
Swelling foundation soil due to rainy periods first lifts the most exposed extremities of the footing system, then the remainder of the perimeter footings while gradually permeating inside the building footprint to lift internal footings. This swelling first tends to create a dish effect, because the external footings are pushed higher than the internal ones.

The first noticeable symptom may be that the floor appears slightly dished. This is often accompanied by some doors binding on the floor or the door head, together with some cracking of cornice mitres. In buildings with timber flooring supported by bearers and joists, the floor can be bouncy. Externally there may be visible dishing of the hip or ridge lines.

As the moisture absorption process completes its journey to the innermost areas of the building, the internal footings will rise. If the spread of moisture is roughly even, it may be that the symptoms will temporarily disappear, but it is more likely that swelling will be uneven, creating a difference rather than a disappearance in symptoms. In buildings with timber flooring supported by bearers and joists, the isolated piers will rise more easily than the strip footings or piers under walls, creating noticeable doming of flooring.

As the weather pattern changes and the soil begins to dry out, the external footings will be first affected, beginning with the locations where the sun's effect is strongest. This has the effect of lowering the

Trees can cause shrinkage and damage



external footings. The doming is accentuated and cracking reduces or disappears where it occurred because of dishing, but other cracks open up. The roof lines may become convex.

Doming and dishing are also affected by weather in other ways. In areas where warm, wet summers and cooler dry winters prevail, water migration tends to be toward the interior and doming will be accentuated, whereas where summers are dry and winters are cold and wet, migration tends to be toward the exterior and the underlying propensity is toward dishing.

Movement caused by tree roots

In general, growing roots will exert an upward pressure on footings, whereas soil subject to drying because of tree or shrub roots will tend to remove support from under footings by inducing shrinkage.

Complications caused by the structure itself

Most forces that the soil causes to be exerted on structures are vertical – i.e. either up or down. However, because these forces are seldom spread evenly around the footings, and because the building resists uneven movement because of its rigidity, forces are exerted from one part of the building to another. The net result of all these forces is usually rotational. This resultant force often complicates the diagnosis because the visible symptoms do not simply reflect the original cause. A common symptom is binding of doors on the vertical member of the frame.

Effects on full masonry structures

Brickwork will resist cracking where it can. It will attempt to span areas that lose support because of subsided foundations or raised points. It is therefore usual to see cracking at weak points, such as openings for windows or doors.

In the event of construction settlement, cracking will usually remain unchanged after the process of settlement has ceased.

With local shear or erosion, cracking will usually continue to develop until the original cause has been remedied, or until the subsidence has completely neutralised the affected portion of footing and the structure has stabilised on other footings that remain effective.

In the case of swell/shrink effects, the brickwork will in some cases return to its original position after completion of a cycle, however it is more likely that the rotational effect will not be exactly reversed, and it is also usual that brickwork will settle in its new position and will resist the forces trying to return it to its original position. This means that in a case where swelling takes place after construction and cracking occurs, the cracking is likely to at least partly remain after the shrink segment of the cycle is complete. Thus, each time the cycle is repeated, the likelihood is that the cracking will become wider until the sections of brickwork become virtually independent.

With repeated cycles, once the cracking is established, if there is no other complication, it is normal for the incidence of cracking to stabilise, as the building has the articulation it needs to cope with the problem. This is by no means always the case, however, and monitoring of cracks in walls and floors should always be treated seriously.

Upheaval caused by growth of tree roots under footings is not a simple vertical shear stress. There is a tendency for the root to also exert lateral forces that attempt to separate sections of brickwork after initial cracking has occurred.

The normal structural arrangement is that the inner leaf of brickwork in the external walls and at least some of the internal walls (depending on the roof type) comprise the load-bearing structure on which any upper floors, ceilings and the roof are supported. In these cases, it is internally visible cracking that should be the main focus of attention, however there are a few examples of dwellings whose external leaf of masonry plays some supporting role, so this should be checked if there is any doubt. In any case, externally visible cracking is important as a guide to stresses on the structure generally, and it should also be remembered that the external walls must be capable of supporting themselves.

Effects on framed structures

Timber or steel framed buildings are less likely to exhibit cracking due to swell/shrink than masonry buildings because of their flexibility. Also, the doming/dishing effects tend to be lower because of the lighter weight of walls. The main risks to framed buildings are encountered because of the isolated pier footings used under walls. Where erosion or saturation causes a footing to fall away, this can double the span which a wall must bridge. This additional stress can create cracking in wall linings, particularly where there is a weak point in the structure caused by a door or window opening. It is, however, unlikely that framed structures will be so stressed as to suffer serious damage without first exhibiting some or all of the above symptoms for a considerable period. The same warning period should apply in the case of upheaval. It should be noted, however, that where framed buildings are supported by strip footings there is only one leaf of brickwork and therefore the externally visible walls are the supporting structure for the building. In this case, the subfloor masonry walls can be expected to behave as full brickwork walls.

Effects on brick veneer structures

Because the load-bearing structure of a brick veneer building is the frame that makes up the interior leaf of the external walls plus perhaps the internal walls, depending on the type of roof, the building can be expected to behave as a framed structure, except that the external masonry will behave in a similar way to the external leaf of a full masonry structure.

Water Service and Drainage

Where a water service pipe, a sewer or stormwater drainage pipe is in the vicinity of a building, a water leak can cause erosion, swelling or saturation of susceptible soil. Even a minuscule leak can be enough to saturate a clay foundation. A leaking tap near a building can have the same effect. In addition, trenches containing pipes can become watercourses even though backfilled, particularly where broken rubble is used as fill. Water that runs along these trenches can be responsible for serious erosion, interstrata seepage into subfloor areas and saturation.

Pipe leakage and trench water flows also encourage tree and shrub roots to the source of water, complicating and exacerbating the problem. Poor roof plumbing can result in large volumes of rainwater being concentrated in a small area of soil:

- Incorrect falls in roof guttering may result in overflows, as may gutters blocked with leaves etc.

- Corroded guttering or downpipes can spill water to ground.
- Downpipes not positively connected to a proper stormwater collection system will direct a concentration of water to soil that is directly adjacent to footings, sometimes causing large-scale problems such as erosion, saturation and migration of water under the building.

Seriousness of Cracking

In general, most cracking found in masonry walls is a cosmetic nuisance only and can be kept in repair or even ignored. The table below is a reproduction of Table C1 of AS 2870-2011.

AS 2870-2011 also publishes figures relating to cracking in concrete floors, however because wall cracking will usually reach the critical point significantly earlier than cracking in slabs, this table is not reproduced here.

Prevention/Cure

Plumbing

Where building movement is caused by water service, roof plumbing, sewer or stormwater failure, the remedy is to repair the problem. It is prudent, however, to consider also rerouting pipes away from the building where possible, and relocating taps to positions where any leakage will not direct water to the building vicinity. Even where gully traps are present, there is sometimes sufficient spill to create erosion or saturation, particularly in modern installations using smaller diameter PVC fixtures. Indeed, some gully traps are not situated directly under the taps that are installed to charge them, with the result that water from the tap may enter the backfilled trench that houses the sewer piping. If the trench has been poorly backfilled, the water will either pond or flow along the bottom of the trench. As these trenches usually run alongside the footings and can be at a similar depth, it is not hard to see how any water that is thus directed into a trench can easily affect the foundation's ability to support footings or even gain entry to the subfloor area.

Ground drainage

In all soils there is the capacity for water to travel on the surface and below it. Surface water flows can be established by inspection during and after heavy or prolonged rain. If necessary, a grated drain system connected to the stormwater collection system is usually an easy solution.

It is, however, sometimes necessary when attempting to prevent water migration that testing be carried out to establish watertable height and subsoil water flows. This subject is referred to in BTF 19 and may properly be regarded as an area for an expert consultant.

Protection of the building perimeter

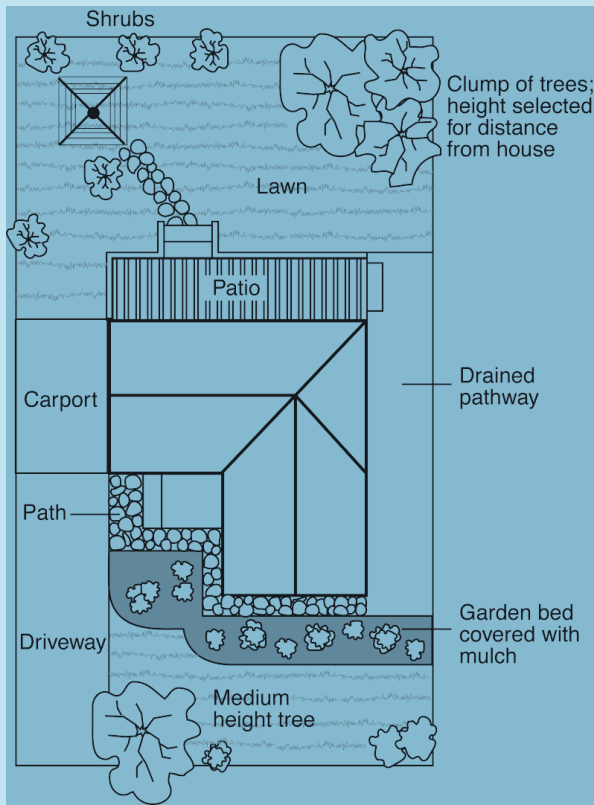
It is essential to remember that the soil that affects footings extends well beyond the actual building line. Watering of garden plants, shrubs and trees causes some of the most serious water problems.

For this reason, particularly where problems exist or are likely to occur, it is recommended that an apron of paving be installed around as much of the building perimeter as necessary. This paving should

CLASSIFICATION OF DAMAGE WITH REFERENCE TO WALLS

Description of typical damage and required repair	Approximate crack width limit (see Note 3)	Damage category
Hairline cracks	<0.1 mm	0
Fine cracks which do not need repair	<1 mm	1
Cracks noticeable but easily filled. Doors and windows stick slightly.	<5 mm	2
Cracks can be repaired and possibly a small amount of wall will need to be replaced. Doors and windows stick. Service pipes can fracture. Weathertightness often impaired.	5–15 mm (or a number of cracks 3 mm or more in one group)	3
Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Window and door frames distort. Walls lean or bulge noticeably, some loss of bearing in beams. Service pipes disrupted.	15–25 mm but also depends on number of cracks	4

Gardens for a reactive site



extend outwards a minimum of 900 mm (more in highly reactive soil) and should have a minimum fall away from the building of 1:60. The finished paving should be no less than 100 mm below brick vent bases.

It is prudent to relocate drainage pipes away from this paving, if possible, to avoid complications from future leakage. If this is not practical, earthenware pipes should be replaced by PVC and backfilling should be of the same soil type as the surrounding soil and compacted to the same density.

Except in areas where freezing of water is an issue, it is wise to remove taps in the building area and relocate them well away from the building – preferably not uphill from it (see BTF 19).

It may be desirable to install a grated drain at the outside edge of the paving on the uphill side of the building. If subsoil drainage is needed this can be installed under the surface drain.

Condensation

In buildings with a subfloor void such as where bearers and joists support flooring, insufficient ventilation creates ideal conditions for condensation, particularly where there is little clearance between the floor and the ground. Condensation adds to the moisture already present in the subfloor and significantly slows the process of drying out. Installation of an adequate subfloor ventilation system, either natural or mechanical, is desirable.

Warning: Although this Building Technology File deals with cracking in buildings, it should be said that subfloor moisture can result in the development of other problems, notably:

- Water that is transmitted into masonry, metal or timber building elements causes damage and/or decay to those elements.
- High subfloor humidity and moisture content create an ideal environment for various pests, including termites and spiders.
- Where high moisture levels are transmitted to the flooring and walls, an increase in the dust mite count can ensue within the living areas. Dust mites, as well as dampness in general, can be a health hazard to inhabitants, particularly those who are abnormally susceptible to respiratory ailments.

The garden

The ideal vegetation layout is to have lawn or plants that require only light watering immediately adjacent to the drainage or paving edge, then more demanding plants, shrubs and trees spread out in that order.

Overwatering due to misuse of automatic watering systems is a common cause of saturation and water migration under footings. If it is necessary to use these systems, it is important to remove garden beds to a completely safe distance from buildings.

Existing trees

Where a tree is causing a problem of soil drying or there is the existence or threat of upheaval of footings, if the offending roots are subsidiary and their removal will not significantly damage the tree, they should be severed and a concrete or metal barrier placed vertically in the soil to prevent future root growth in the direction of the building. If it is not possible to remove the relevant roots without damage to the tree, an application to remove the tree should be made to the local authority. A prudent plan is to transplant likely offenders before they become a problem.

Information on trees, plants and shrubs

State departments overseeing agriculture can give information regarding root patterns, volume of water needed and safe distance from buildings of most species. Botanic gardens are also sources of information. For information on plant roots and drains, see Building Technology File 17.

Excavation

Excavation around footings must be properly engineered. Soil supporting footings can only be safely excavated at an angle that allows the soil under the footing to remain stable. This angle is called the angle of repose (or friction) and varies significantly between soil types and conditions. Removal of soil within the angle of repose will cause subsidence.

Remediation

Where erosion has occurred that has washed away soil adjacent to footings, soil of the same classification should be introduced and compacted to the same density. Where footings have been undermined, augmentation or other specialist work may be required. Remediation of footings and foundations is generally the realm of a specialist consultant.

Where isolated footings rise and fall because of swell/shrink effect, the homeowner may be tempted to alleviate floor bounce by filling the gap that has appeared between the bearer and the pier with blocking. The danger here is that when the next swell segment of the cycle occurs, the extra blocking will push the floor up into an accentuated dome and may also cause local shear failure in the soil. If it is necessary to use blocking, it should be by a pair of fine wedges and monitoring should be carried out fortnightly.

This BTF was prepared by John Lewer FAIB, MIAMA, Partner, Construction Diagnosis.

The information in this and other issues in the series was derived from various sources and was believed to be correct when published.

The information is advisory. It is provided in good faith and not claimed to be an exhaustive treatment of the relevant subject.

Further professional advice needs to be obtained before taking any action based on the information provided.

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 **Sorell Council**

Development Application: 5.2025.194.1 -
Development Application 64 Friendship Drive,
Sorell - P1.pdf

Plans Reference:P1
Date Received:23/07/2025

CORROSION ENVIRONMENT: N/A

The documentation listed below should be read in conjunction with these drawings and form the basis of construction documentation for the project

[illegible]

GENERAL NOTES:

1. THIS PLAN HAS BEEN PREPARED BY SURVEY PLUS FROM A COMBINATION OF EXISTING RECORDS AND FIELD SURVEY FOR THE PURPOSES OF SHOWING THE PHYSICAL FEATURES OF THE LAND AND SHOULD NOT BE USED FOR ANY OTHER PURPOSE.

2. TITLE BOUNDARIES SHOWN WERE NOT VERIFIED OR MARKED BY SURVEY PLUS AT THE TIME OF THIS SURVEY.

3. SERVICES SHOWN ON THIS PLAN WERE LOCATED WHERE POSSIBLE BY FIELD SURVEY. THEY ARE NOT A COMPLETE PICTURE OF SERVICES ON SITE. ALL SERVICE LOCATIONS ARE TO BE VERIFIED BEFORE COMMENCEMENT OF ANY WORK ON SITE, IN PARTICULAR THOSE SERVICES NOT PREVIOUSLY LOCATED THROUGH FIELD SURVEY.

4. SURVEY PLUS CAN NOT ACCEPT LIABILITY WHATSOEVER FOR LOSS OR DAMAGE CAUSED TO ANY UNDERGROUND SERVICE WHETHER SHOWN BY OUR SURVEY OR NOT.

5. THIS NOTE IS AN INTEGRAL PART OF THIS PLAN/DATA. REPRODUCTION OF THIS PLAN OR ANY PART OF IT WITHOUT THIS NOTE BEING INCLUDED IN FULL WILL RENDER THE INFORMATION SHOWN ON SUCH A REPRODUCTION INVALID AND NOT SUITABLE FOR USE WITHOUT PRIOR AUTHORITY OF SURVEY PLUS.

6. HORIZONTAL DATUM IS MGA (GDA94).

7. VERTICAL DATUM IS AHD.

8. CONTOUR INTERVAL IS 0.2 METRE, INDEX IS 1.0 METRE.

9. SURVEY BY ROBOTIC TOTAL STATION AND GPS.

10. IMPORTED DATA SHOWN ON THIS PLAN WAS OBTAINED FOR PUBLIC AVAILABLE DATA FROM VARIOUS GOVERNMENT AUTHORITIES. THIS INFORMATION IS PROVIDED FOR GUIDANCE ONLY. THE ACCURACY OF ANY IMPORTED DATA IS PER THE ACCURACY QUOTED BY THE SOURCE AND IS IN NO WAY GUARANTEED BY SURVEY PLUS. USERS MUST NOT RELY ON THIS DATA FOR ON-GROUND LOCATION OF BOUNDARIES AND/OR SERVICES.

LIST DATA IMPORT

- TasWater-SewerLateralLine
- TasWater-SewerMain
- TasWater-SewerMaintenanceHole
- TasWater-SewerPressurisedMain
- TasWater-WaterHydrant
- TasWater-WaterLateralLine
- TasWater-WaterMain

11. BOUNDARIES ARE COMPILED ONLY FROM SP185351 AND RELEVANT SURVEY INFORMATION OBTAINED FROM LAND TITLES OFFICE AND ARE APPROXIMATE AND SUBJECT TO SURVEY.

12. ALL WINDOWS WERE NOT ABLE TO BE LOCATED DUE TO OBSTRUCTION OF LINE OF SIGHT FROM TOTAL STATION. WINDOW LOCATIONS ARE APPROXIMATE ONLY DUE TO BEING UNABLE TO BE PERPENDICULAR TO WINDOWS WHEN LOCATING WITH TOTAL STATION.

13. 3D DATA TURNED OFF IN LAYER CONTROL.

- 3D TIN
- MAJOR CONTOUR 3D
- MINOR CONTOUR 3D

•DP 90mm DOWNPIPE

NOTE: ALL PROPOSED STORMWATER TO BE DISCHARGED TO EXISTING INFRASTRUCTURE

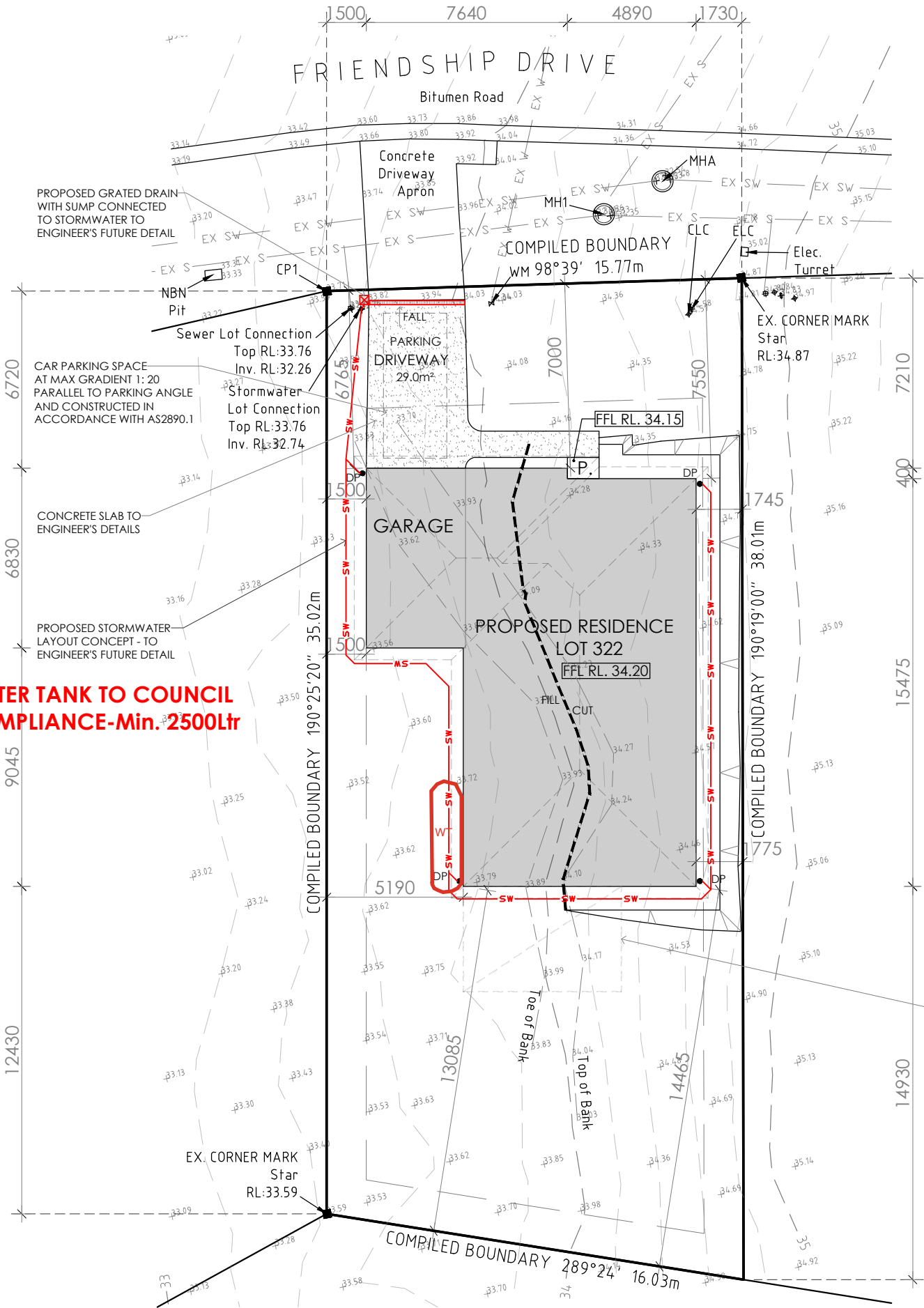
EROSION AND SEDIMENT CONTROL MEASURES ARE TO BE IMPLEMENTED ON THE SITE IN ACCORDANCE WITH COUNCIL REQUIREMENTS

PROPOSED SITE PLAN

© COPYRIGHT IN WHOLE OR IN PART

02	PROJECT NORTH	CONTRACTOR MUST VERIFY ALL DIMENSIONS AND LEVELS AT THE JOB PRIOR TO COMMENCING ANY WORK OR MAKING ANY SHOP DRAWINGS.		REV:	DESCRIPTION:	BY:	DATE:
		DO NOT SCALE DRAWINGS. ALWAYS USE WRITTEN DIMENSIONS.		SK	ISSUED FOR CLIENT REVIEW	NN	17/6/25
				SK1	REDESIGN FLOOR PLAN	NN	12/7/25
				SK2	LEAVE THE GARAGE, FLIP THE RESIDENCE	NN	14/7/25
				A	ISSUED FOR CLIENT REVIEW	NN	22/7/25

PRELIMINARY



- W1 - 1.22W x 0.85H Top RL:34.62 Ground Floor Window
D2 - 0.85W x 0.21H Top RL:32.90 Ground Floor Door
W3 - 1.82W x 1.20H Top RL:34.62 Ground Floor Window
W4 - 0.62W x 1.47H Top RL:34.62 Ground Floor Window
W5 - 0.62W x 1.47H Top RL:34.62 Ground Floor Window
W6 - 0.62W x 1.47H Top RL:34.62 Ground Floor Window
W7 - 0.62W x 1.47H Top RL:34.62 Ground Floor Window

LEGEND

ELC = Electricity Lot Connection
CLC = Communication Lot Connection

CP1 = EX. CORNER MARK
Star
RL:33.71

MH1 = Sewer Manhole
Top RL:34.36
In Inv RL:32.33
Out Inv RL:32.31

MHA = Stormwater Manhole
Top RL:34.53
In Inv RL:32.54
In Inv RL:32.48
Out Inv RL:32.31

GPS DATA SCALE LOCATION
JOB CONTROL POINT
POINT NO: @@
DESCRIPTION: R/S IN KERB#
GPS SCALE FACTOR = 1.000378447
E: 547344.861
N: 5263724.246
RL: 37.41
EPU = 0.04±

24m² PRIMARY PRIVATE OPEN SPACE (DASHED LINE) WITH MINIMUM 4M DIMENSION, MAX 1:10 SLOPE (LEVEL AS REQUIRED) IN ACCORDANCE WITH COUNCIL REQUIREMENTS



Development Application: 5.2025.194.1 -
Development Application 64 Friendship Drive,
Sorell - P1.pdf
Plans Reference:P1
Date Received:23/07/2025

NOTES

SITE PREPARATION
THE SITE IS TO BE DISTURBED AS MINIMALLY AS POSSIBLE TO THE EXTENT REQUIRED TO CARRY OUT THE BUILDING WORKS.
EARTHWORKS SHALL BE CARRIED OUT IN ACCORDANCE WITH NCC PART 3.2.
UN-RETAINED EMBANKMENT GRADIENTS SHALL BE IN ACCORDANCE WITH NCC TABLE 3.2.1.
DRAINAGE SHALL BE IN ACCORDANCE WITH NCC PART 3.3.2.
THE BUILDER AND SUBCONTRACTOR SHALL ENSURE THAT ALL STORMWATER DRAINS, SEWER PIPES AND THE LIKE ARE LOCATED AT A SUFFICIENT DISTANCE FROM ANY BUILDINGS FOOTING AND/OR SLAB EDGE BEAMS SO AS TO PREVENT GENERAL MOISTURE PENETRATION, DAMPNES, WEAKENING & UNDERMINING OF ANY BUILDING AND ITS FOOTING SYSTEM.
LOCATION OF ALL EXISTING SERVICES TO BE CONFIRMED ON SITE PRIOR TO CONSTRUCTION.
ATTENTION OF OWNER
THE OWNERS ATTENTION IS DRAWN TO THE FACT THAT FOUNDATIONS AND ASSOCIATED DRAINAGE FOR ALL SITES REQUIRES CONTINUING MAINTENANCE TO ASSIST FOOTING PERFORMANCE. ADVICE FOR FOUNDATION MAINTENANCE IS CONTAINED IN THE CSIRO BUILDING TECHNOLOGY FILE 18 AND IT IS THE OWNERS RESPONSIBILITY TO MAINTAIN THE SITE IN ACCORDANCE WITH THIS DOCUMENT.
SOIL AND WATER MANAGEMENT NOTES:
DRAINAGE LINES ARE TO BE INSTALLED PRIOR TO THE PLACEMENT OF ROOF AND GUTTERING. ONCE DWELLING IS ROOFED, CONNECT IMMEDIATELY. APPLY TEMPORARY COVERING TO DISTURBED AREAS THAT WILL REMAIN EXPOSED FOR 14 DAYS OR MORE DURING CONSTRUCTION (EG. WATERPROOF BLANKET, VEGETATION OR MULCH)
PROTECT ANY NEARBY OR ON-SITE DRAINAGE PITS FROM SEDIMENT BY INSTALLING SEDIMENT TRAPS AROUND THEM.
LIMIT ENTRY/EXIT TO ONE POINT AND STABILISE. INSTALL FACILITIES TO REMOVE DIRT/ MUD FROM VEHICLE WHEELS BEFORE THEY LEAVE THE SITE.
SITE TO BE VEGETATED AND PLANTED ACCORDING TO THE HOBART REGIONAL SOIL AND WATER MANAGEMENT CODE OF PRACTICE.
BUILDER AND SUBCONTRACTORS TO VERIFY ALL DIMENSIONS AND LEVELS PRIOR TO THE COMMENCEMENT OF ANY WORK. GIVE 24 HOURS MINIMUM NOTICE WHERE AMENDMENTS ARE REQUIRED TO DRAWINGS. THESE DRAWINGS ARE TO BE READ IN CONJUNCTION WITH DOCUMENTATION LISTED ON THE COVER PAGE. DO NOT SCALE DRAWINGS.
DIMENSIONS ARE TO TAKE PREFERENCE OVER SCALE. BUILDING SPECIFICATION AND ENGINEERS DRAWINGS SHALL OVERRIDE ARCHITECTURAL DRAWINGS.
-THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL OTHER DRAWING SHEETS, CONSULTANTS DRAWINGS, DOCUMENTS, SCHEDULES AND SPECIFICATIONS (AS APPLICABLE).
-THE BUILDER AND SUBCONTRACTOR SHALL ENSURE THAT ALL STORMWATER DRAINS, SEWER PIPES AND THE LIKE ARE LOCATED AT A SUFFICIENT DISTANCE FROM ANY BUILDINGS FOOTING AND/OR SLAB EDGE BEAMS SO AS TO PREVENT GENERAL MOISTURE PENETRATION, DAMPNES, WEAKENING & UNDERMINING OF ANY BUILDING AND ITS FOOTING SYSTEM.
- LOCATION OF ALL EXISTING ONSITE SERVICES TO BE CONFIRMED ONSITE PRIOR TO CONSTRUCTION

IMPORTANT!
SITE INFORMATION AS DRAWN IS APPROXIMATE ONLY. FINAL SITE INFORMATION IS SUBJECT TO A DETAILED CONTOUR SURVEY BY LICENSED SURVEYOR.

SOIL CLASSIFICATION:	--
WIND CLASSIFICATION:	--
SITE COVERAGE	
SITE AREA	576.8 m²
PROPOSED BUILDING FOOTPRINT	164.5 m²
PROPOSED SITE COVERAGE	28.53 %

CREATIVE HOMES HOBART, CNR OF ELWICK ROAD & BROOKER HIGHWAY, GLENORCHY 7010 PH: 03 6272 3000

JOB ADDRESS: 322 Friendship Drive Sorell		CLIENT: Greg Rothacker	
DESIGNER: I. Brown	ACCRED. NO.: CC6652	SHEET:	2 of 9
DRAWN:N. Nguyen	DATE: June 2025	DESIGN TYPE:	Custom
CHECKED:	DATE:	DRAWING NO:	---
SCALE: 1:200	REV: A		

AREAS:	
FLOOR AREA:	137.8 m ²
PORCH:	1.0 m ²
GARAGE:	25.8 m ²
TOTAL AREA: 164.5 m ²	

LEGEND:	
DP	DOWNPIPE LOCATION
SHR	SHOWER
BTH	BATH
VB	VANITY BASIN
WC	TOILET
OHC	OVERHEAD CUPBOARDS
REF	REFRIGERATOR
P	PANTRY
RH	RANGE HOOD
UBO	UNDER BENCH OVEN
CT	COOK TOP
S	SINK
DW	DISH WASHER
T	TROUGH
WM	WASHING MACHINE
MH	MANHOLE
CSD	CAVITY SLIDING DOOR
AAW	ALUM. AWNING WINDOW
AFW	ALUM. FIXED WINDOW
ASD	ALUM. SLIDING DOOR
B/O	BEAM OVER
BAL	BALUSTRADE

BAL SELECTED BALUSTRADE TO BUILDER'S STANDARD DETAIL INSTALLED IN ACCORDANCE WITH NCC

MASONRY ARTICULATION JOINT - LOCATION TO ENGINEER'S DETAILS

CEILING MOUNTED INTERCONNECTED SMOKE DETECTORS, MAINS WIRED WITH BATTERY BACKUP, ALL IN ACCORDANCE WITH AS 3786.

DP 90mm DOWNPIPE

NOTE: LIFT OFF HINGES TO WC TO BE INSTALLED AS REQUIRED IN ACCORDANCE WITH NCC.

PROPOSED FLOOR PLAN

FLOOR AREA: 164.5m²

PRELIMINARY

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03



CONTRACTOR MUST VERIFY ALL DIMENSIONS AND LEVELS AT THE JOB PRIOR TO COMMENCING ANY WORK OR MAKING ANY SHOP DRAWINGS.

DO NOT SCALE DRAWINGS. ALWAYS USE WRITTEN DIMENSIONS.

REV:	DESCRIPTION:	BY:	DATE:
SK	ISSUED FOR CLIENT REVIEW	NN	17/6/25
SK1	REDESIGN FLOOR PLAN	NN	12/7/25
SK2	LEAVE THE GARAGE, FLIP THE RESIDENCE	NN	14/7/25
A	ISSUED FOR CLIENT REVIEW	NN	22/7/25



CREATIVE HOMES
HOBART

CREATIVE HOMES HOBART, CNR OF ELWICK ROAD & BROOKER HIGHWAY, GLENORCHY 7010 PH: 03 6272 3000

JOB ADDRESS: 322 Friendship Drive Sorell		CLIENT: Greg Rothacker
DESIGNER: I. Brown	ACCRED. NO.: CC6652	SHEET: 3 of 9
DRAWN: N. Nguyen	DATE: June 2025	DESIGN TYPE: Custom
CHECKED:	DATE:	DRAWING NO: ---
SCALE: 1:100	REV: A	



WATER TANK TO COUNCIL COMPLIANCE-Min. 2500ltr

Framing NCC H1D6
All timber framing, fixing and bracing shall comply with AS 1684 and the requirements of NCC H1D6. Manufactured sizes must not be undersized to those specified, for all timber sizes, stress grades, spacing and wall bracing refer to Engineer's details. Tie-down details shall be in accordance with Engineer's details and comply with NCC H1D6 (4). Structural steel members shall comply with the requirements of clauses in NCC H1D6 (3). Refer to Engineer's details where provided.

Glazing NCC H1D8
All windows to be aluminium awning style, double glazed (obscured safety glass to bathrooms as shown on drawings) All glazing shall comply with the requirements of AS 2047 & AS 1288 and NCC H1D8.

Human impact safety requirements shall comply with NCC H1D8 (3) and Part 8.4 of the ABCB Housing provisions.

Note:
Builder and subcontractors to verify all dimension and levels prior to the commencement of any work. Give 24hrs minimum notice where amendments are required to design of working drawings. These drawings are to be read in conjunction with Engineer's and Surveyor's drawings and notes. Do not scale drawings. Dimensions are to take preference over scale. Building specification and Engineer's drawings shall override architectural drawings. All construction work shall be carried out in accordance with the state building regulations, local council by-laws and relevant NCC and AS codes.

Important notice for attention of Owners:
the Owners attention is drawn to the fact that foundations and associated drainage in all sites requires continuing maintenance to assist footing performance. Advice for foundation maintenance is contained in the CSIRO building technology file 18 and it is the Owners responsibility to maintain the site in accordance with this document.

Energy efficiency:
Insulation must comply with AS/NZS4859.1 and be installed in accordance with ABCB housing provisions Part 13.2.2 and comply with minimum R values for climate zone 7.
Bulk insulation between external studs to be insulated with min R2.0. (Ensure batts fit within cavity without compression, making sure that there is at least 25mm gap from the reflective surface). External walls are to be clad with vapour permeable reflective foil over the outside of the timber frame. Ceiling to be insulated with R4.0 and vapour permeable sarking. Floor to be insulated with Min R1.7 batts where applicable. Seal exhaust fans to Ensuite, Bathroom, Laundry and Kitchen. All downlights to be IC rated. Construction of the external walls, floor and roof for compliance with building sealing requirements shall comply with BCA 2019 Part 3.12

General:
All flashings, weep holes and damp proof coursing to be in accordance with NCC Housing provisions Part 5.7. Fibre cement sheet in accordance with NCC Housing provisions Part 7.5. Block construction in accordance NCC Housing provisions Part 5. Plasterboard linings to internal walls and ceilings with selected cornice. (see below for wet areas)

Wet areas: All wet areas shall comply with the requirements of ABCB Housing provisions Part 10.2. Provide waterproof plasterboard sheeting to all walls and ceilings. Provide ceramic tiles or other approved water resistant lining in accordance with Part 10.2.9 to a minimum height of 1800mm to shower walls and to a height of min 150mm behind baths, basins, sinks, troughs, washing machines and wall fixtures.

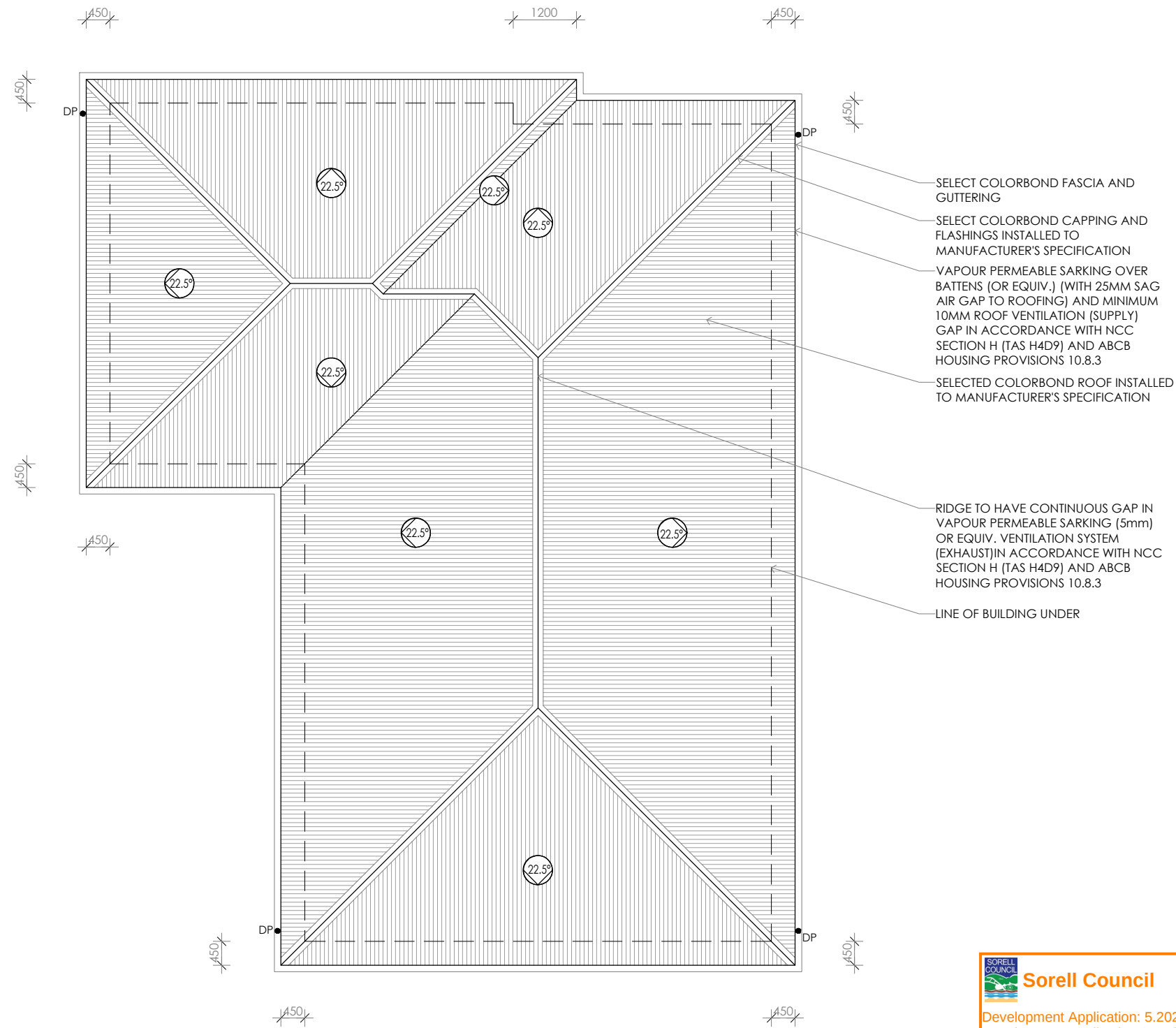
For construction of floor wastes refer to NCC ABCB Housing provisions part 10.2.12. For typical installation requirements for substrate preparation, penetrations, flashings/junctions, membranes, screeds, hobs, baths, showers, door jambs and screens refer to ABCB Housing provisions part 10.2.14-32.



Sorell Council

Development Application: 5.2025.194.1 -
Development Application 64 Friendship Drive,
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WATER TANK TO COUNCIL
COMPLIANCE-Min. 2500Ltr



Framing NCC H1D6
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Glazing NCC H1D8
All windows to be aluminium awning style, double glazed (obscured safety glass to bathrooms as shown on drawings) All glazing shall comply with the requirements of AS 2047 & AS 1288 and NCC H1D8.

Human impact safety requirements shall comply with NCC H1D8 (3) and Part 8.4 of the ABCB Housing provisions.

Note:
Builder and subcontractors to verify all dimension and levels prior to the commencement of any work. Give 24hrs minimum notice where amendments are required to design of working drawings. These drawings are to be read in conjunction with Engineer's and Surveyor's drawings and notes. Do not scale drawings. Dimensions are to take preference over scale. Building specification and Engineer's drawings shall override architectural drawings. All construction work shall be carried out in accordance with the state building regulations, local council by-laws and relevant NCC and AS codes.

Important notice for attention of Owners: the Owners attention is drawn to the fact that foundations and associated drainage in all sites requires continuing maintenance to assist footing performance. Advice for foundation maintenance is contained in the CSIRO building technology file 18 and it is the Owners responsibility to maintain the site in accordance with this document.

Energy efficiency:
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Sorell Council
Development Application: 5.2025.194.1 -
Development Application 64 Friendship Drive,
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PROPOSED ROOF PLAN

PRELIMINARY

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			SK1	REDESIGN FLOOR PLAN	NN	12/7/25
			SK2	LEAVE THE GARAGE, FLIP THE RESIDENCE	NN	14/7/25
			A	ISSUED FOR CLIENT REVIEW	NN	22/7/25

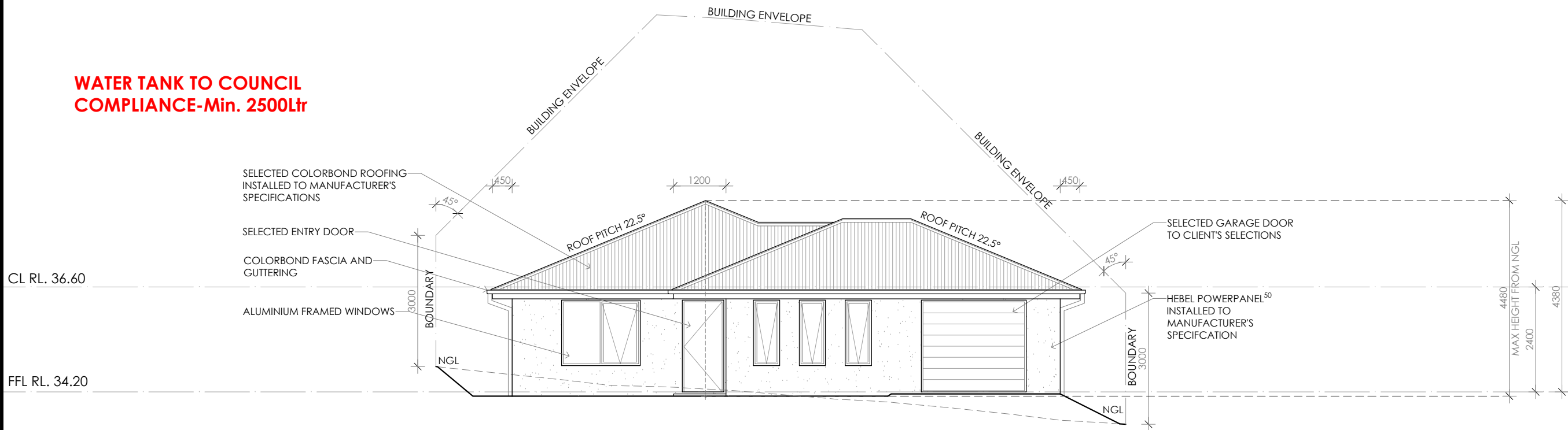


**CREATIVE HOMES
HOBART**

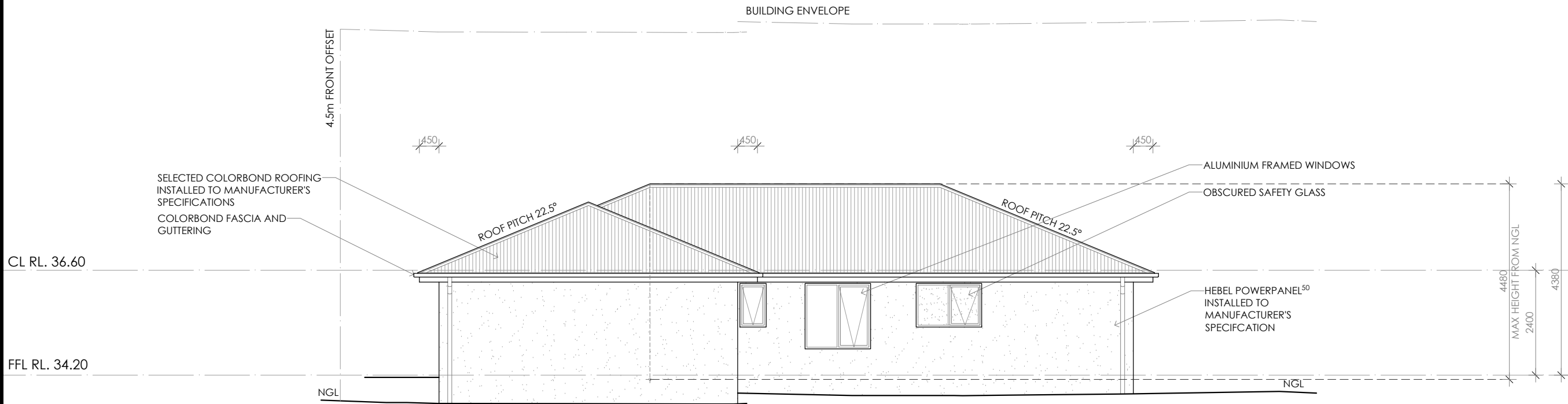
CREATIVE HOMES HOBART, CNR OF ELWICK ROAD & BROOKER HIGHWAY, GLENORCHY 7010 PH: 03 6272 3000

JOB ADDRESS: 322 Friendship Drive Sorell		CLIENT: Greg Rothacker	
DESIGNER: I. Brown	ACCRED. NO.: CC6652	SHEET: 4 of 9	
DRAWN:N. Nguyen	DATE: June 2025	DESIGN TYPE: Custom	
CHECKED:	DATE:	DRAWING NO: ---	
SCALE: 1:100	REV: A		

WATER TANK TO COUNCIL
COMPLIANCE-Min. 2500Ltr



NORTHERN ELEVATION



WESTERN ELEVATION

Framing NCC H1D6
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**Sorell Council**

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Development Application 64 Friendship Drive,
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Plans Reference:P1
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REV:	DESCRIPTION:	BY:	DATE:
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A	ISSUED FOR CLIENT REVIEW	NN	22/7/25

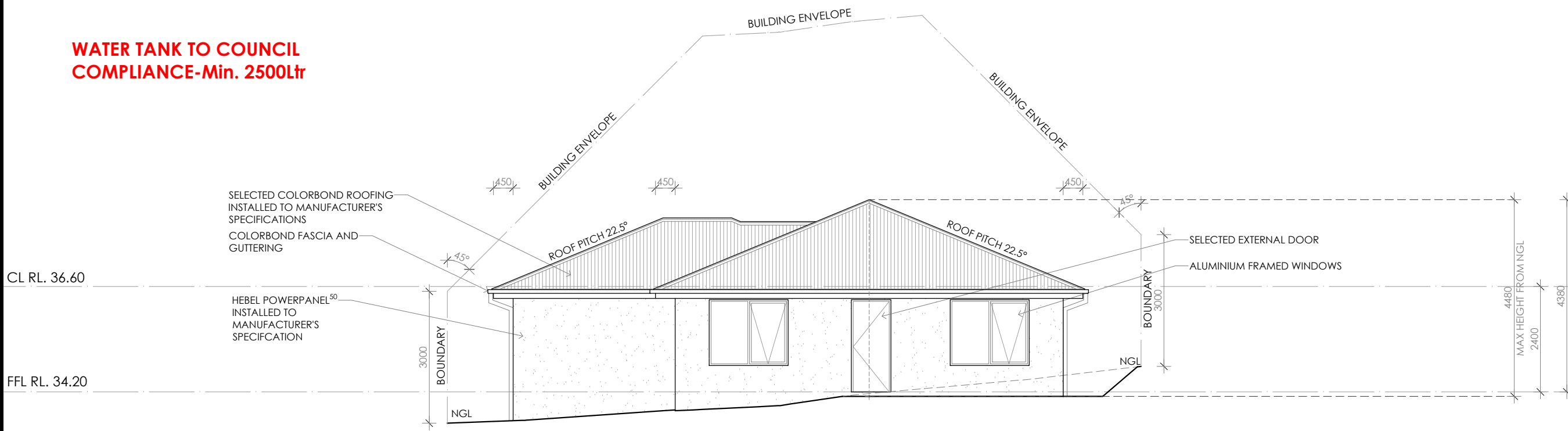
PRELIMINARY

**CREATIVE HOMES
HOBART**

CREATIVE HOMES HOBART, CNR OF ELWICK ROAD & BROOKER HIGHWAY, GLENORCHY 7010 PH: 03 6272 3000

JOB ADDRESS: 322 Friendship Drive Sorell		CLIENT: Greg Rothacker	
DESIGNER: I. Brown	ACCRED. NO.: CC6652	SHEET: 5 of 9	
DRAWN:N. Nguyen	DATE: June 2025	DESIGN TYPE: Custom	
CHECKED:	DATE:	DRAWING NO: ---	
SCALE: 1:100	REV: A		

WATER TANK TO COUNCIL
COMPLIANCE-Min. 2500Ltr



SOUTHERN ELEVATION



EASTERN ELEVATION

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**Sorell Council**

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PROJECT NORTH

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A	ISSUED FOR CLIENT REVIEW	NN	22/7/25

**CREATIVE HOMES
HOBART**

CREATIVE HOMES HOBART, CNR OF ELWICK ROAD & BROOKER HIGHWAY, GLENORCHY 7010 PH: 03 6272 3000

JOB ADDRESS:		CLIENT:
322 Friendship Drive Sorell		Greg Rothacker
DESIGNER: I. Brown	ACCRED. NO.: CC6652	SHEET: 6 of 9
DRAWN:N. Nguyen	DATE: June 2025	DESIGN TYPE: Custom
CHECKED:	DATE:	DRAWING NO: ---
SCALE: 1:100	REV: A	

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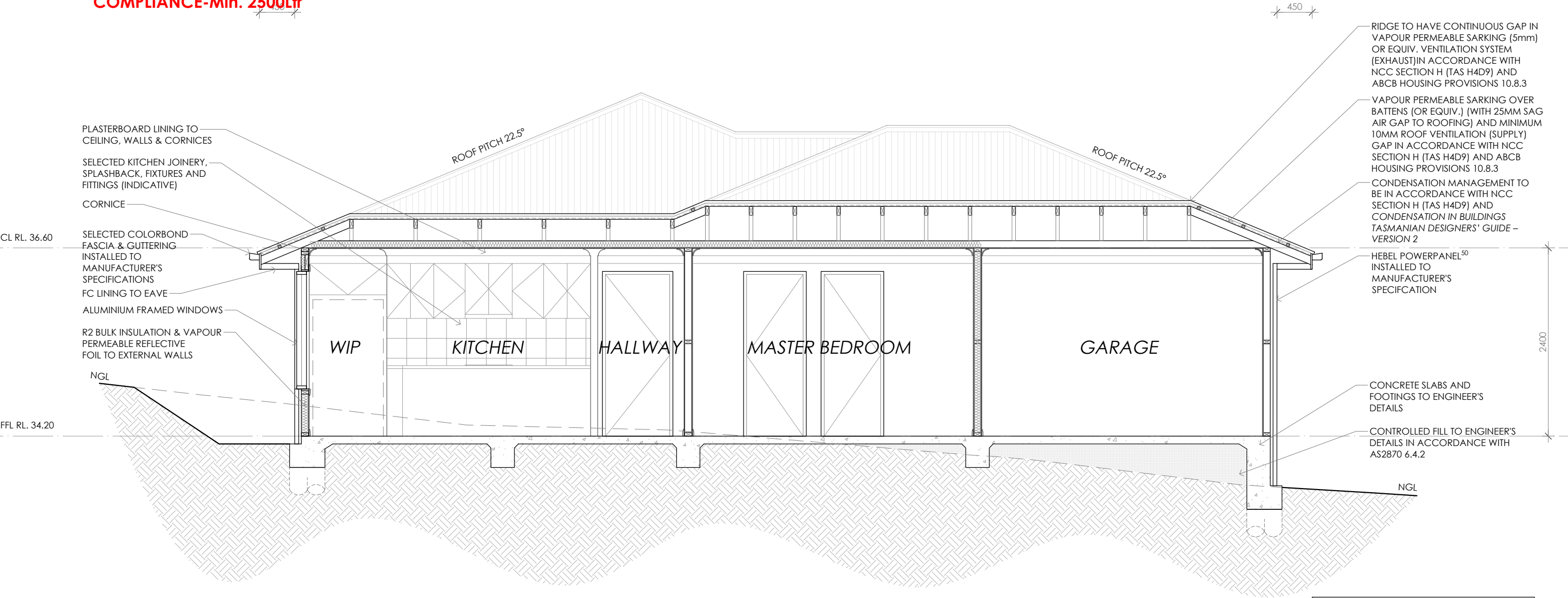
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COMPLIANCE-Min. 2500Ltr



TRUSS MANUFACTURER TO NOTE:
ALL TRUSSES TO BE DESIGNED TO BE SUPPORTED BY ALL EXTERNAL WALLS FOR BOTH DEAD AND WIND LOADS.

SECTION A-A

PRELIMINARY

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SK2	LEAVE THE GARAGE, FLIP THE RESIDENCE	NN	14/7/25
A	ISSUED FOR CLIENT REVIEW	NN	22/7/25

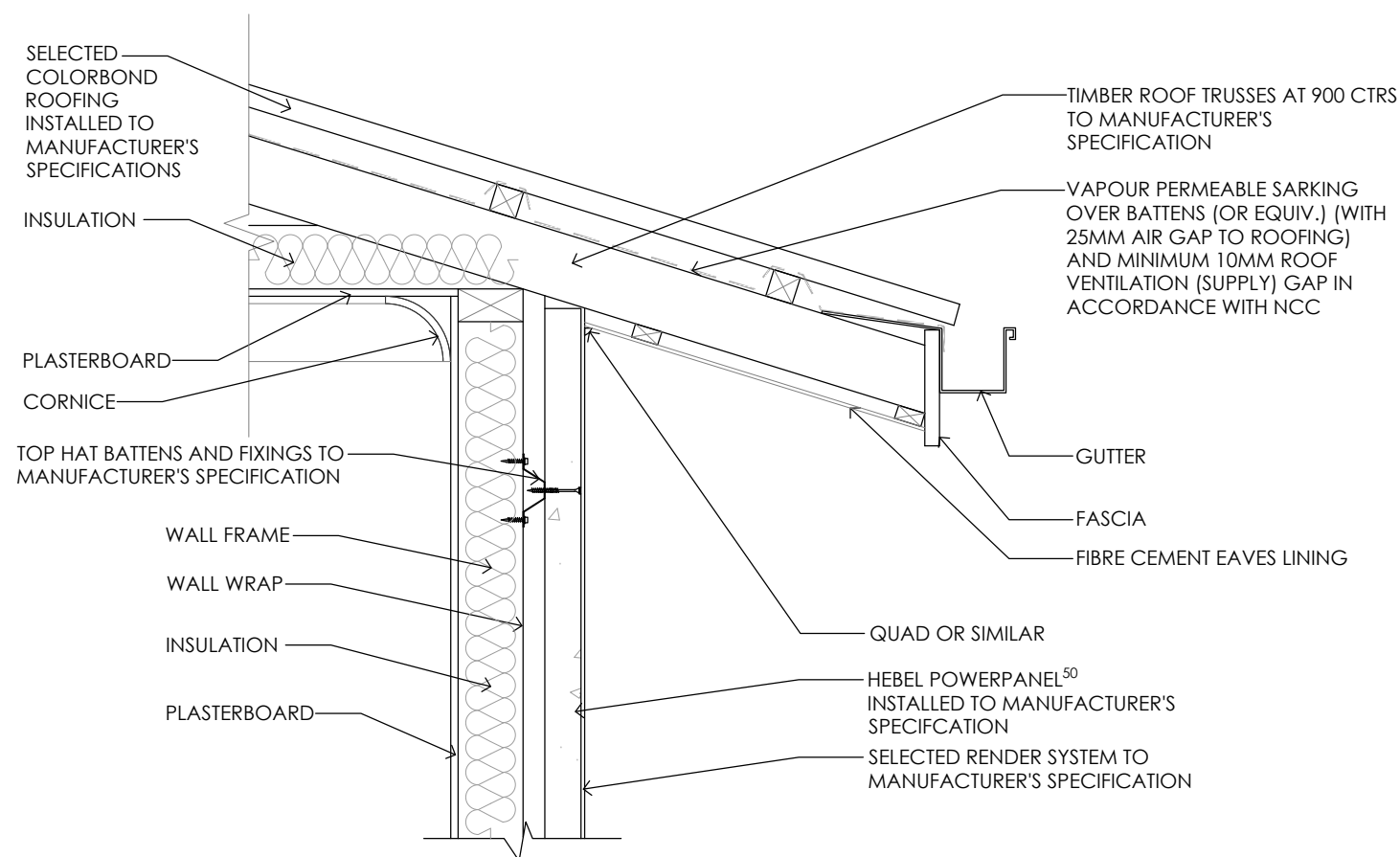


**CREATIVE HOMES
HOBART**

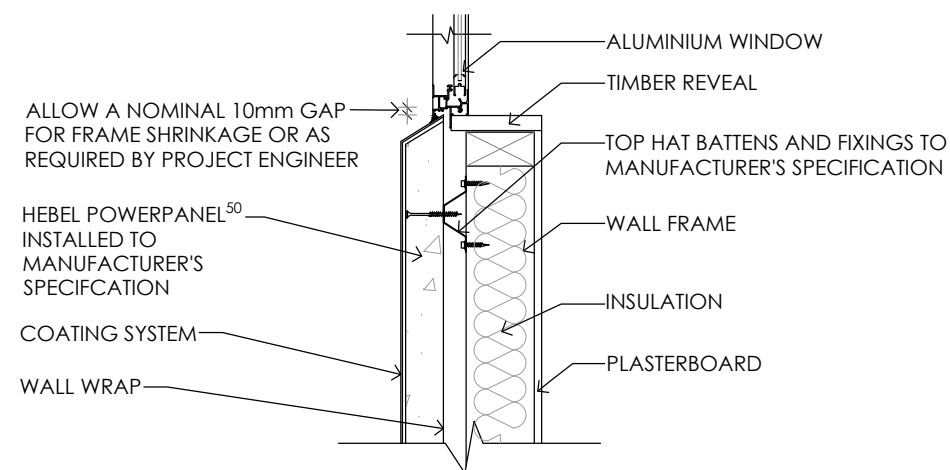
CREATIVE HOMES HOBART, CNR OF ELWICK ROAD & BROOKER HIGHWAY, GLENORCHY 7010 PH: 03 6272 3000

JOB ADDRESS: 322 Friendship Drive Sorell		CLIENT: Greg Rothacker	
DESIGNER: I. Brown	ACCRED. NO.: CC6652	SHEET:	7 of 9
DRAWN:N. Nguyen	DATE: June 2025	DESIGN TYPE:	Custom
CHECKED:	DATE:	DRAWING NO:	---
SCALE: 1:50	REV: A		

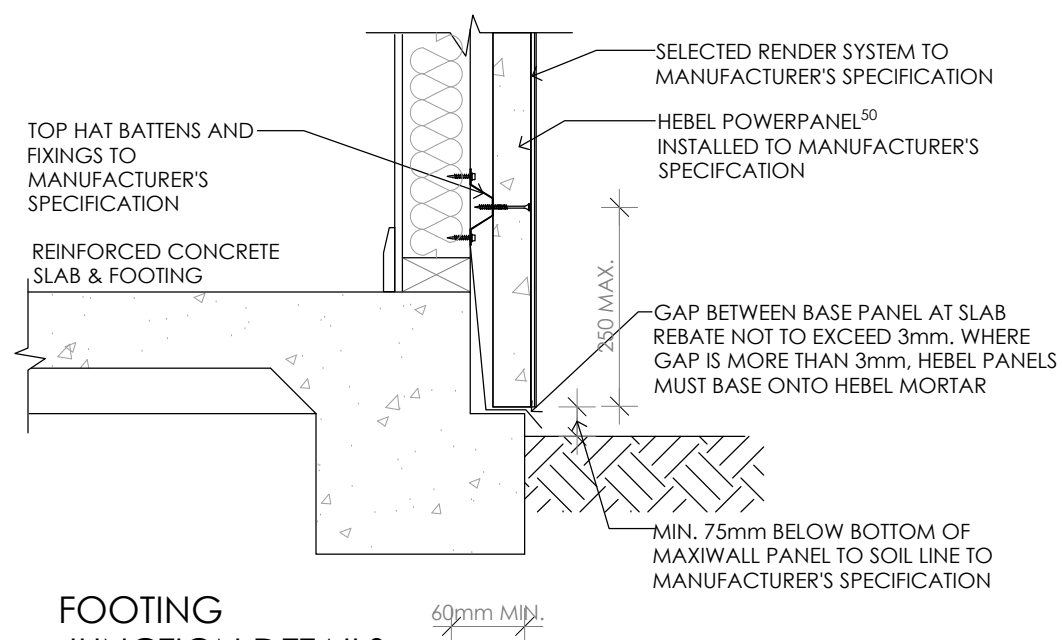
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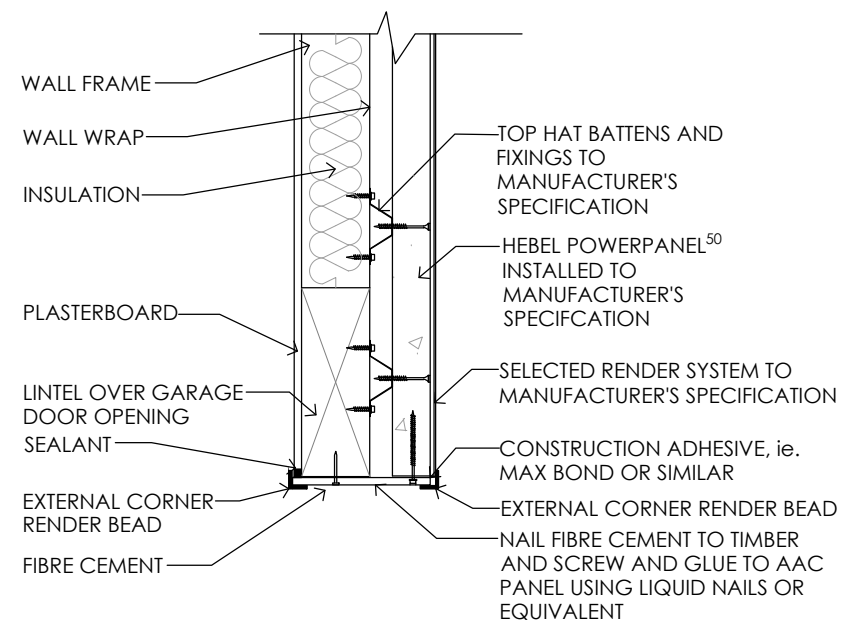
TYPICAL ROOF EAVES DETAILS



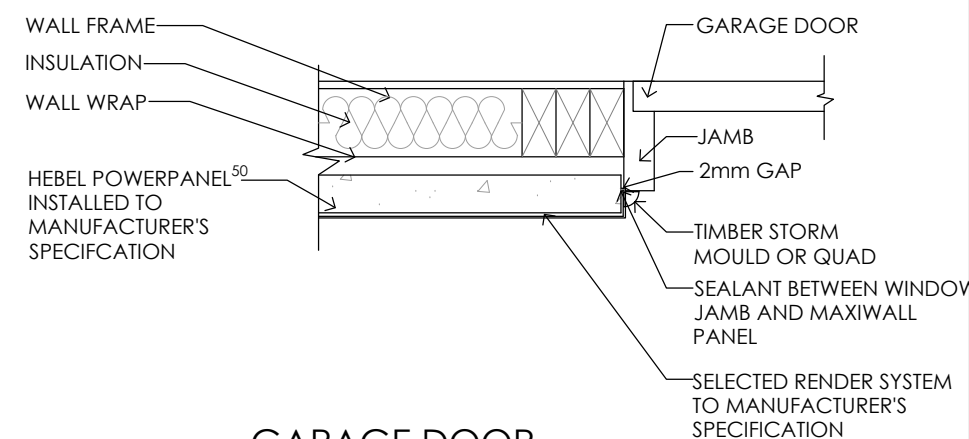
TYPICAL WINDOW SILL DETAIL ALUMINIUM WINDOW FRAME



FOOTING JUNCTION DETAILS

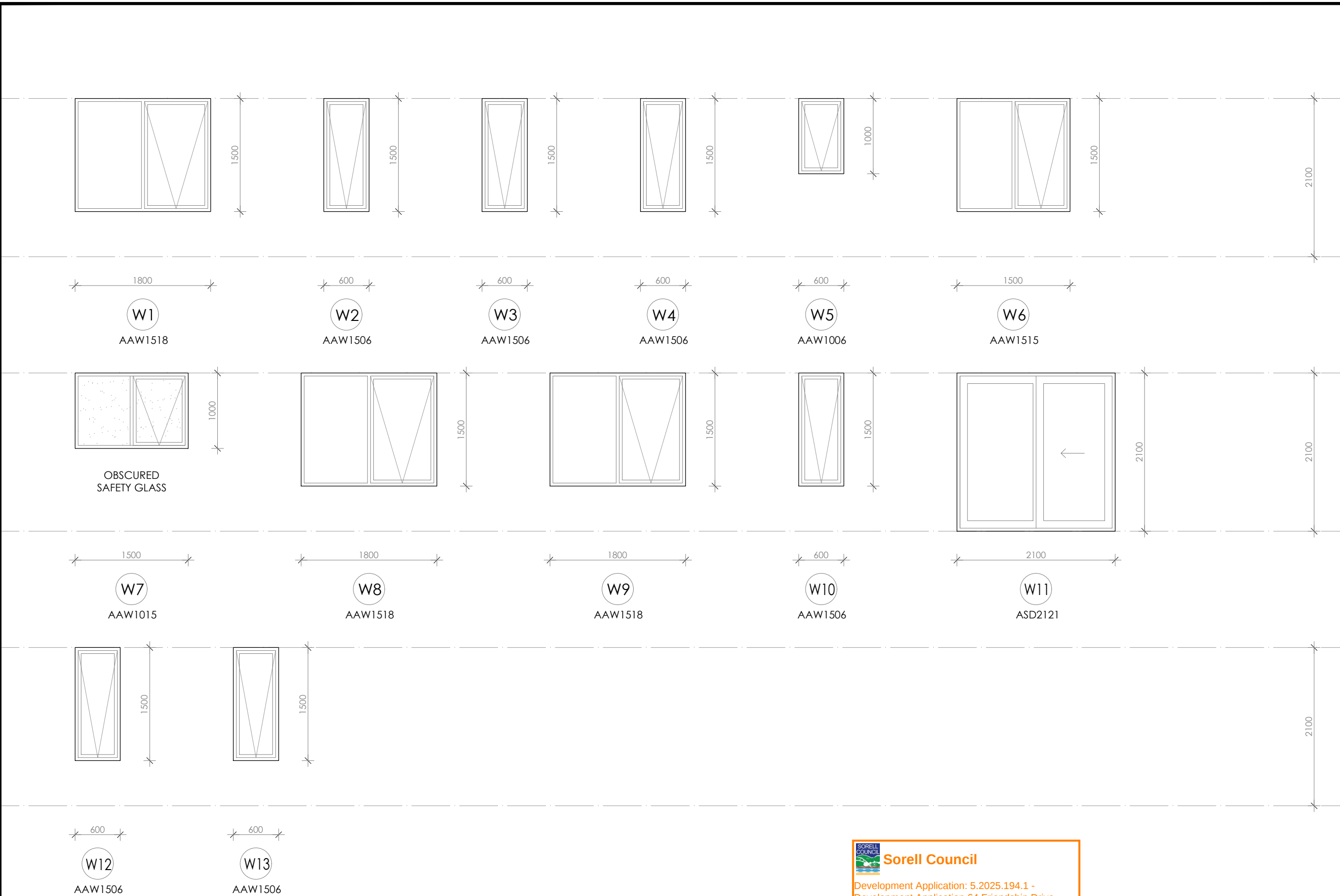


GARAGE DOOR - HEAD DETAIL



GARAGE DOOR - JAMB DETAIL

PRELIMINARY



WINDOW SCHEDULE



Sorell Council
Development Application: 5.2025.194.1 -
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			A	ISSUED FOR CLIENT REVIEW	NN	22/7/25							